

# Matrix Force Method

## 1- Trusses

Compatibility Condition

4

$$\frac{\Delta}{(n \times 1)} + \frac{F}{(n \times n)} \Delta_{(n \times 1)} = \frac{p}{(n \times 1)}$$

Where

$$\frac{\Delta}{(n \times 1)} = \frac{Nq}{(n \times m)} \cdot \int u \cdot \frac{Np}{(m \times 1)} \rightarrow \Delta$$

and the flexibility matrix  $F_{(n \times n)}$

$$F_{(n \times n)} = \frac{Nq}{(n \times m)} \cdot \int u \cdot \frac{Nq}{(m \times m)}$$

in 2 get  $\Delta_{(n \times 1)}$   
 in 3 get  $F_{(n \times n)}$   
 in 1 get  $X_{(n \times 1)}$

$$N_{(m \times 1)} = Np_{(m \times 1)} + \frac{Nq}{(m \times n)} X_{(n \times 1)}$$

matrix N.f.

5

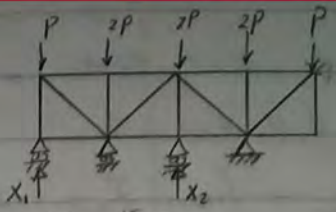
- $Nq$  = d.f. in truss members due to unit load
- $Np$  = normal force in truss members due to applied loads
- $\Delta$  = displacement due to external loads
- $F$  = flexibility matrix
- $F_{ij}$  = displacement at point  $i$  due to unit load at point  $j$

$X$  = unknowns (forces required  $X_1, X_2, \dots, X_n$ )  
 $N$  = Normal force "required"  
 $n$  = degree of indeterminacy  $\rightarrow$  statical  
 $m$  = Number of members in truss

6

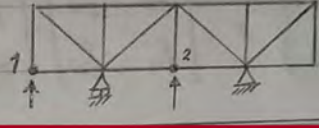
$$F_{(n \times n)} = \begin{bmatrix} \frac{L}{EA} & 0 & \dots & 0 \\ 0 & \frac{L}{EA} & \dots & 0 \\ \vdots & \vdots & \ddots & \vdots \\ 0 & 0 & \dots & \frac{L}{EA} \end{bmatrix}_{m \times m}$$

1

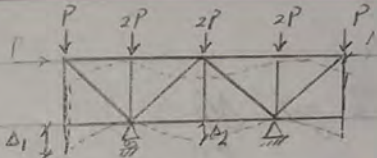


$J=10, m=17, r=5$   
 $n=17+5-2 \times 10 \rightarrow n=2$

Coordinates of primary system



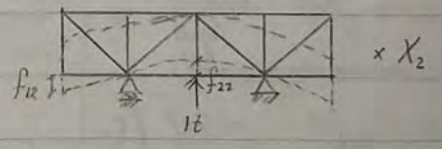
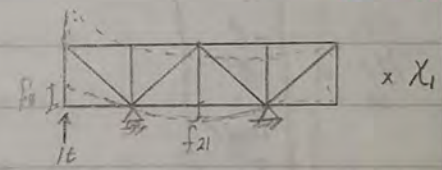
$\Delta$  due to external loads



$$Np = \begin{bmatrix} \dots \\ \dots \\ \dots \\ \dots \\ \dots \end{bmatrix}_{m \times 1}$$

2

$$Nq_{(m \times n)} = \begin{bmatrix} \dots \\ \dots \\ \dots \\ \dots \\ \dots \end{bmatrix}$$



on primary sys

3

$$\Delta_1 + f_{11} X_1 + f_{12} X_2 = 0$$

$$\Delta_2 + f_{21} X_1 + f_{22} X_2 = 0$$

$$\begin{Bmatrix} \Delta_1 \\ \Delta_2 \end{Bmatrix} + \begin{bmatrix} f_{11} & f_{12} \\ f_{21} & f_{22} \end{bmatrix} \begin{Bmatrix} X_1 \\ X_2 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \end{Bmatrix}$$

$n=2$

$$\frac{\Delta}{(n \times 1)} + F_{(n \times n)} \Delta_{(n \times 1)} = \frac{p}{(n \times 1)}$$

= flexibility matrix for unassembled structure

# EX-01 Truss; from 2016-2017 Final Exam - Modal Answer

2/5

L4

10

Final normal forces in the members

$$\begin{Bmatrix} -1.2778 \\ 1.9841 \\ -3.7083 \\ -2.1528 \\ 1.2698 \end{Bmatrix} t$$

$$N = N_p + N_q X =$$

$$2J = 8$$

1

2

Problem

$$m + r = 5 + 5 = 10$$

Primary system

3

4-5

$$N_p = \begin{Bmatrix} -3 \\ -3.67 \\ -5 \\ 0 \\ 8.333 \end{Bmatrix} t$$

$$N_q = \begin{bmatrix} 1.333 & 0 \\ 1.333 & 1 \\ 1.0 & 0 \\ -1.667 & 0 \\ -1.667 & -1.25 \end{bmatrix}$$

$$F_u = \frac{1}{10500} \begin{bmatrix} 4 & 0 & 0 & 0 & 0 \\ 0 & 4 & 0 & 0 & 0 \\ 0 & 0 & 3 & 0 & 0 \\ 0 & 0 & 0 & 5 & 0 \\ 0 & 0 & 0 & 0 & 5 \end{bmatrix}$$

$$L_{211} = N_q^T F_u N_p = \begin{Bmatrix} -0.0114 \\ -0.0089 \end{Bmatrix}$$

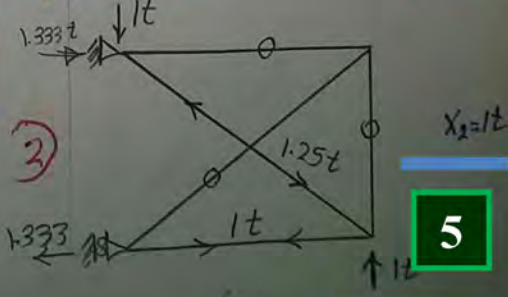
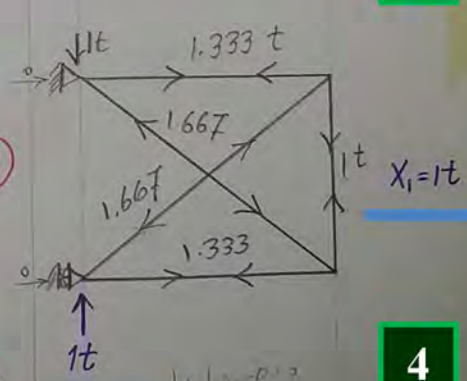
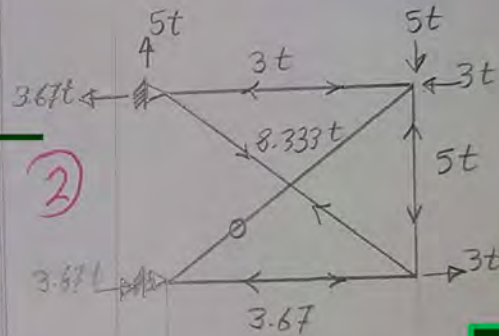
8

9

$$F_{u2} = N_q^T F_u N_q = \begin{bmatrix} 0.0043 & 0.0020 \\ 0.0020 & 0.0020 \end{bmatrix}$$

$$X = \begin{Bmatrix} 1.2917 \\ 2.9464 \end{Bmatrix} t$$

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EX-02 Truss

$J=4, m=6, r=4$

$n = 6 + 4 - 2 \times 4 = 2$

$\Delta_{(n \times 1)} + \underline{F}_{(2 \times 2)} \underline{X}_{(2 \times 1)} = \underline{0}_{(2 \times 1)}$

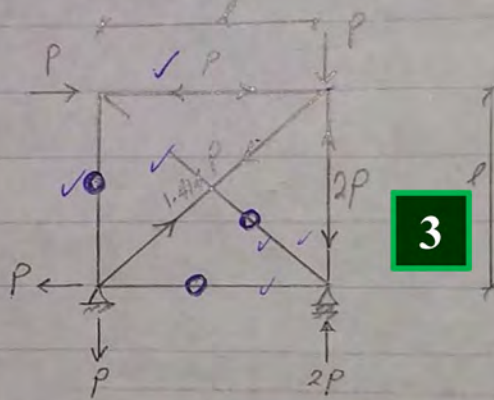
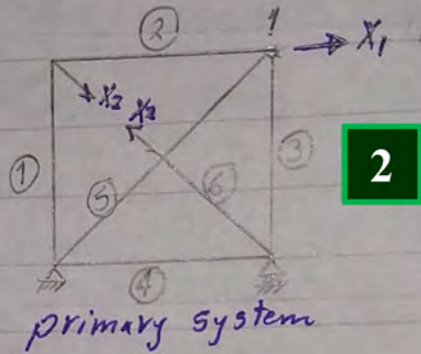
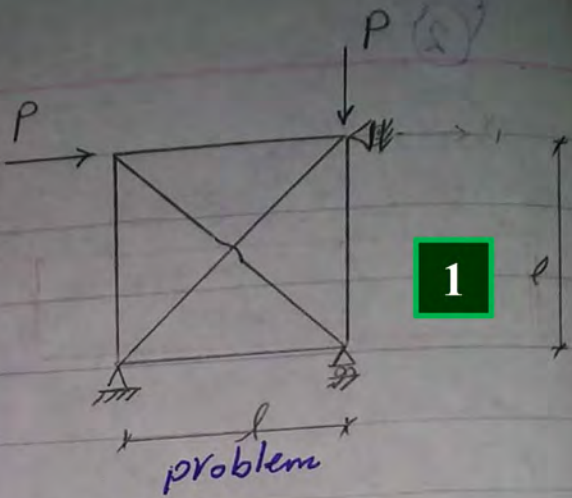
$\Delta_{(2 \times 2)} + \underline{F}_{(2 \times 2)} \underline{X}_{(2 \times 1)} = \underline{0}_{(2 \times 1)}$

$\begin{Bmatrix} \Delta_1 \\ \Delta_2 \end{Bmatrix} + \begin{bmatrix} f_{11} & f_{12} \\ f_{21} & f_{22} \end{bmatrix} \begin{Bmatrix} X_1 \\ X_2 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \end{Bmatrix}$

To get  $\underline{N}_p$   
 $\begin{matrix} m \times 1 \\ 6 \times 1 \end{matrix}$

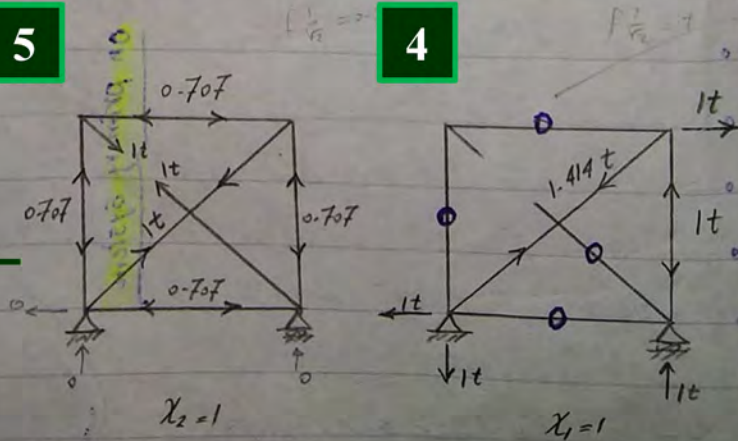
$\underline{N}_p = \begin{Bmatrix} 0 \\ -P \\ -2P \\ 0 \\ 1.414P \\ 0 \end{Bmatrix}$

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To get  $\underline{N}_q$   
 $\begin{matrix} m \times n \\ 6 \times 2 \end{matrix}$

$\underline{N}_q = \begin{Bmatrix} 0 & -0.707 \\ 0 & -0.707 \\ -1 & -0.707 \\ 0 & -0.707 \\ 1.414 & 1 \\ 0 & 1 \end{Bmatrix}$   
 $X_1=1 \quad X_2=1$



# EX-02 Truss

4/5

L4

To get  $F_{u_{6 \times 6}}$

$$F_{u_{6 \times 6}} = \frac{L}{EA}$$

$$\begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1.414 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1.414 \end{bmatrix}$$

6

To get  $\Delta_{n \times 1}$

$$\Delta_{(n \times 1)} = N_q^T \cdot F_{u_{(m \times m)}} \cdot N_p$$

$$\Delta_{(2 \times 1)} = N_q^T \cdot F_{u_{(6 \times 6)}} \cdot N_p$$

7

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$$\begin{Bmatrix} \Delta_1 \\ \Delta_2 \end{Bmatrix} = \frac{L}{EA} \begin{bmatrix} 0 & 0 & -1 & 0 & 1.414 & 0 \\ -0.707 & -0.707 & -0.707 & -0.707 & 1.414 & 1.414 \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 0 & 1.414 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1.414 \end{bmatrix} N_p$$

$\frac{EA}{L} \cdot N_{x_1} = 1$ 
 $\frac{EA}{L} \cdot N_{x_2} = 1$

$$= \frac{PL}{EA} \begin{bmatrix} 0 & 0 & -1 & 0 & 2 & 0 \\ -0.707 & -0.707 & -0.707 & -0.707 & 1.414 & 1.414 \end{bmatrix} \begin{Bmatrix} 0 \\ -1 \\ -2 \\ 0 \\ 1.414 \\ 0 \end{Bmatrix}$$

$$\begin{Bmatrix} \Delta_1 \\ \Delta_2 \end{Bmatrix} = \frac{PL}{EA} \begin{Bmatrix} 4.828 \\ 4.121 \end{Bmatrix}$$

To get  $F_{2 \times 2}$  the flexibility matrix

$$F_{max} = N_q^T \cdot F_u \cdot N_q$$

$(n \times m)$        $(m \times m)$        $(m \times n)$

8

$$F_{(2 \times 2)} = N_q^T \cdot F_u \cdot N_q$$

$(2 \times 6)$        $(6 \times 6)$        $(6 \times 2)$

$\delta_{11}$        $\delta_{12}$   
 $\delta_{21}$        $\delta_{22}$

$$\begin{bmatrix} f_{11} & f_{12} \\ f_{21} & f_{22} \end{bmatrix} = \frac{L}{EA} \begin{bmatrix} 0 & 0 & -1 & 0 & 2 & 0 \\ -0.707 & -0.707 & -0.707 & -0.707 & 1.414 & 1.414 \end{bmatrix}$$

$L$  طول المثلث

$$\begin{bmatrix} 0 & -0.707 \\ 0 & -0.707 \\ -1 & -0.707 \\ 0 & -0.707 \\ 1.414 & 1 \\ 0 & 1 \end{bmatrix}$$

$$\begin{bmatrix} f_{11} & f_{12} \\ f_{21} & f_{22} \end{bmatrix} = \frac{L}{EA} \begin{bmatrix} 3.828 & 2.707 \\ 2.707 & 4.828 \end{bmatrix}$$

\* Compatibility Condition

$$\Delta_{2 \times 1} + F_{2 \times 2} X_{2 \times 1} = 0_{2 \times 1}$$

9

$$\frac{PL}{EA} \begin{bmatrix} 4.828 \\ 4.121 \end{bmatrix} + \frac{L}{EA} \begin{bmatrix} 3.828 & 2.707 \\ 2.707 & 4.828 \end{bmatrix} \begin{bmatrix} X_1 \\ X_2 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \end{bmatrix}$$

$$\begin{bmatrix} X_1 \\ X_2 \end{bmatrix} = - \frac{PL \times EA}{EA \times A} \frac{1}{(3.828)(4.828) - (2.707)^2} \begin{bmatrix} 4.828 & -2.707 \\ -2.707 & 3.828 \end{bmatrix} \begin{bmatrix} 4.828 \\ 4.121 \end{bmatrix}$$

$$\begin{bmatrix} X_1 \\ X_2 \end{bmatrix} = \begin{bmatrix} -1.091 P \\ -0.243 P \end{bmatrix}$$

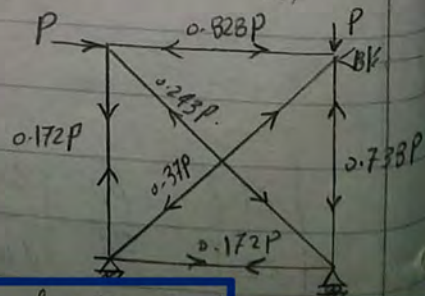
$$N_{(max)} = N_p + N_q X_{(min)}$$

Final normal forces in the members

$$\begin{bmatrix} N_1 \\ N_2 \\ N_3 \\ N_4 \\ N_5 \end{bmatrix} = \begin{bmatrix} 0 \\ -P \\ -2P \\ 0 \\ 1.414P \end{bmatrix} + \begin{bmatrix} 0 & -0.707 \\ 0 & -0.707 \\ -1 & -0.707 \\ 0 & -0.707 \\ 1.414 & 1 \end{bmatrix} \begin{bmatrix} -1.091 P \\ -0.243 P \end{bmatrix} = \begin{bmatrix} 0.172 P \\ -0.828 P \\ -0.738 P \\ 0.172 P \end{bmatrix}$$

10

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## Annex

### Solution for

### matlab & SAP for Question of Truss by Force Method

By *DR. Ahmed M. EL-Kholy*

NP =

-3.0000  
-3.6667  
-5.0000  
0  
8.3333

NQ =

1.3333 0  
1.3333 1.3333  
1.0000 0  
-1.6667 0  
-1.6667 -1.6667

FU =

1.0e-03 \*

0.3810 0 0 0 0  
0 0.3810 0 0 0  
0 0 0.2857 0 0  
0 0 0 0.4762 0  
0 0 0 0 0.4762

D =

-0.0114  
-0.0085

F =

0.0043 0.0020  
0.0020 0.0020

X =

1.2917  
2.9464

N =

-1.2778  
1.9841  
-3.7083  
-2.1528  
1.2698

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